M.J. Davenport & Associates Ltd.

CONSULTING ENGINEERS AND PLANNERS

MURRAY J. DAVENPORT, P.Eng.

MICHAEL M. DAVENPORT, P.Eng.

October 12, 2022

Township of Douro-Dummer 894 South Street, P.O. Box 92 Warsaw, ON KOL 3A0

Attention:

Ms. Christina Coulter

Senior Planner

Re:

Paterson and Carrington - Site Plan Approval 4034 Centre Road, Bolton's Corners

Township of Douro-Dummer

Project No. 21-D-5883

Dear Christina,

Attached is a copy of the following drawings and calculations prepared in response to the D.M. Wills Associates comments dated October 4, 2022 and the Otonabee Region Conservation Authority comments dated October 5, 2022 submitted for your information:

- 1. Site Plan Drawing No. 5883-02.
- 2. Drainage Area Plan Drawing No. 5883-03.
- 3. Erosion Control Plan Drawing No. 5883-EC.
- 4. 100-year Rational Method and Open Channel Flow Calculation for Proposed Drainage Ditch.

D.M. Wills Associates Comments:

1. The revision block should be updated to reflect the current submission and date.

The revision blocks on the Site Plan and the Erosion Control Plan have been updated.

2. Confirm the drainage area and 100-year flow directed to the proposed outfall ditch. A rough sketch and rational method calculations are sufficient to address this comment.

Drainage Area Plan Drawing No. 5883-03 identifies the total catchment area and the landuse of the area flowing into the proposed outfall ditch. The 100-year rational method calculations and the open channel flow calculations attached to this letter indicates that the proposed outfall ditch has sufficient capacity to convey peak stormwater flows in excess of the calculated 100-year flow rates.

3. The size and depth of rip-rap should be identified on the Site Plan.

150mm diameter rip-rap with a design depth of 0.30 metres.

4. An updated Erosion Control Plan is required and should include the external ditching.

The Erosion Control Plan has been updated to show the external ditching. Light duty silt fence is proposed to be placed around the proposed outfall ditch and the culvert to be removed and replaced. A 0.20-metre-high pea stone bag flow check dam is proposed in the outfall ditch and will remain in place until the soils are stabilized and the outfall ditch vegetation is established.

Otonabee Region Conservation Authority Comments:

- 1. Please add rip-rap Slope Protection details to the drawing.
 - a. What size of rip-rap is being placed?
 - b. What thickness of rip-rap layer?
 - c. There should be a scour protection measure placed under the proposed rip-rap?

The specified rip-rap is a 0.30 metre depth of 150mm diameter rip-rap. The rip rap will be placed on non-woven terrafix filter cloth for scour protection.

2. Please provide re-vegetation details for the ditch and slope.

A note has been added to the Site Plan specifying that the proposed drainage ditch, side slopes and disturbed area around the drainage ditch shall be hydroseeded after final grading has been completed. Hydroseeding will establish and stabilize the underlying soil more quickly then seed and requires less initial maintenance than sod.

3. Please include erosion and sediment control measured for the ditch and slope work.

J. D. CLARK 100226373

See response to D.M. Wills Associates comment Point #4 above.

We are available to discuss this project at your convenience.

Yours truly,

J.D. O. T. M.J. DAVENPORT & ASSOCIATES LTD,

Jacob Clark, P.Eng.

Ms. Kathy Carrington & Mr. David Paterso c.c.:

M.J. Davenport & Associates Ltd.

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PATERSON & CARRINGTON 4034 Center Road, Bolton's Corners PROPOSED DITCH RATIONAL METHOD CALCULATION

The peak flow capacity of the grass ditch proposed to be constructed northwest of the project site was analyzed using the Manning's equation for calculating open channel flow. The Rational Method was used to calculate the 100-year storm event peak flow rate expected to accumulate in the ditch based on the upstream catchment area. By demonstrating that the conveyance capacity of the proposed drainage ditch is greater than the calculated 100-year peak flow rate, it will be shown that adequate capacity of conveyance in the proposed ditch exists.

The watershed area directed to the proposed drainage ditch is based on the natural topography of the land determined by a topographical survey, by a visual field inspection and from Ontario Base Mapping taken from the County of Peterborough GIS application. The proposed grading of the subject site is used in determining the ditch drainage area. The watershed area draining into the proposed ditch is detailed on the Drainage Area Plan Drawing No. 5883-03.

The proposed drainage ditch is designed with a minimum flow depth of 0.47 metres, maximum 5:1 side slopes and a minimum longitudinal slope of 2.0 percent. It is assumed in the following calculations that the ditch lining will consist of unmaintained tall grass, making the 'n' value in the Manning's equation 0.050. The conveyance capacity of the proposed ditch has been evaluated at the section with the lowest depth of flow and the narrowest bottom section. This corresponds to the section of the ditch with the lowest conveyance capacity.

The detailed ditch capacity calculation using the Manning's equation are as follows:

Worst Case Grass Ditch Section (0.47m Depth, 3.0m Bottom Width):

$$Q = \frac{1.0}{n} A R^{\frac{2}{3}} S^{\frac{1}{2}}$$

$$= \left(\frac{1.0}{0.050}\right) (2.5145) (0.3227)^{\frac{2}{3}} (0.020)^{\frac{1}{2}}$$

$$= 3.346 m^{3} / s$$

n = 0.050 (Manning's coefficient for unmaintained tall grass lined channels)

 $A = 2.5145m^2$ (Area of cross-section of channel)

R = 0.3227m (Hydraulic radius)

S = 2.00% m/m (Lowest slope of proposed swale)

Rational Method – 100-Year Peak Flow Rate Calculations

Proposed Drainage Ditch:

The drainage area directed to the proposed ditch is approximately 1.679 hectares. The watershed length is 230 metres and the average watershed slope is 1.6%. The calculated time of concentration is 11.3 minutes using the Bransby-Williams Method for the watershed area. The minimum time of concentration is conservatively assumed to be 10 minutes for the urban subwatershed area. The peak flow rate expected to accumulate in the drainage ditch during the 100-year storm event is calculated using the Rational Method as follows:

$$Q = 0.0028CiA$$

= 0.0028 × 0.41 × 141.1 × 1.679
= 0.272 m^3/s

Q = Peak runoff rate (m³/s)

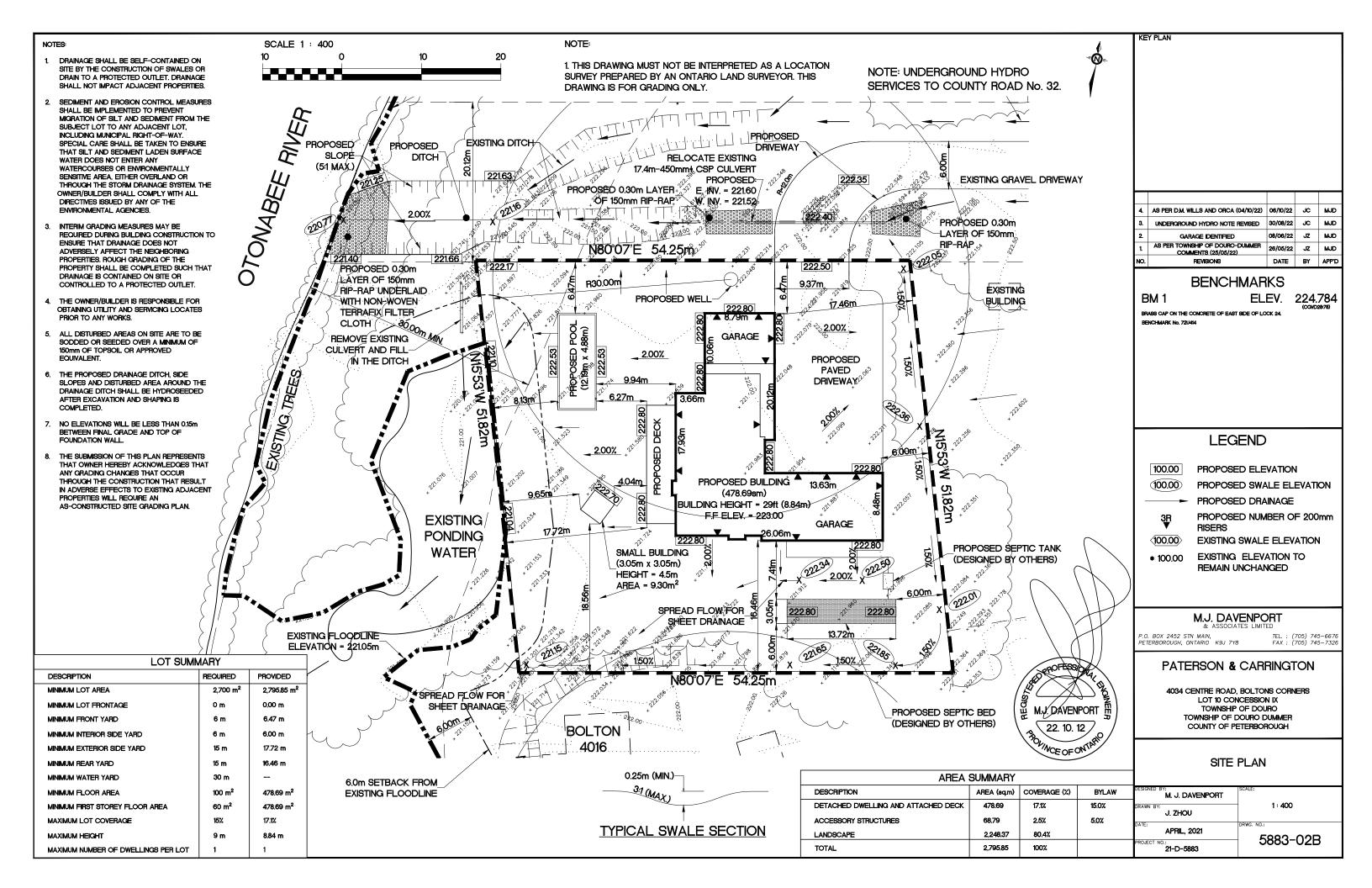
C = 0.41 (Weighted composite runoff coefficient of drainage area - Table 1)

i = 141.1 mm/hr (10 minute - 100 Year Peterborough Airport rainfall intensity, mm/hr)

A = 1.679 ha (Drainage area, ha)

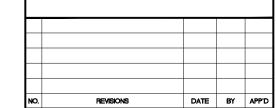
Table 1 Drainage Ditch Weighted Composite Runoff Coefficient				
Surface	Area (sm)	Runoff Coefficient (C)		
Impervious	2,104	0.90		
Grass Landscaped	9,937	0.25		
Gravel	2,285	0.80		
Treed	2,467	0.25		
Total	16,793	0.41		

From the calculations demonstrated above, the proposed drainage ditch will provide adequate capacity of conveyance of calculated peak flow rates generated by all storm events up to and including the 100-year storm event.



NOTES: 1. THE WATERSHED AREA DIRECTED TO THE PROPOSED OUTLET DITCH IS BASED ON THE NATURAL TOPOGRAPHY OF THE LAND DETERMINED BY A TOPOGRAPHICAL SURVEY CARRIED OUT BY M.J. DAVENPORT + ASSOCIATES, A VISUAL FIELD INSPECTION AND FROM ONTARIO BASE MAPPING FROM THE COUNTY OF PETERBOROUGH GIS SYSTEM. THE PROPOSED GRADING OF THE PROJECT SITE WAS ACCOUNTED FOR IN THE WATERSHED AREA DETERMINATION.





BENCHMARKS

BM 1

KEY PLAN

ELEV. 224.784

BRASS CAP ON THE CONCRETE OF EAST SIDE OF LOCK 24. BENCHMARK No. 72U414

LEGEND

PROPOSED ELEVATION 100.00 PROPOSED SWALE ELEVATION

DRAINAGE

DRAINAGE AREA BOUNDARY

100.00 EXISTING SWALE ELEVATION

* 100.00 EXISTING ELEVATION TO

REMAIN UNCHANGED

P.O. BOX 2452 STN MAIN, PETERBOROUGH, ONTARIO K9J 7Y8

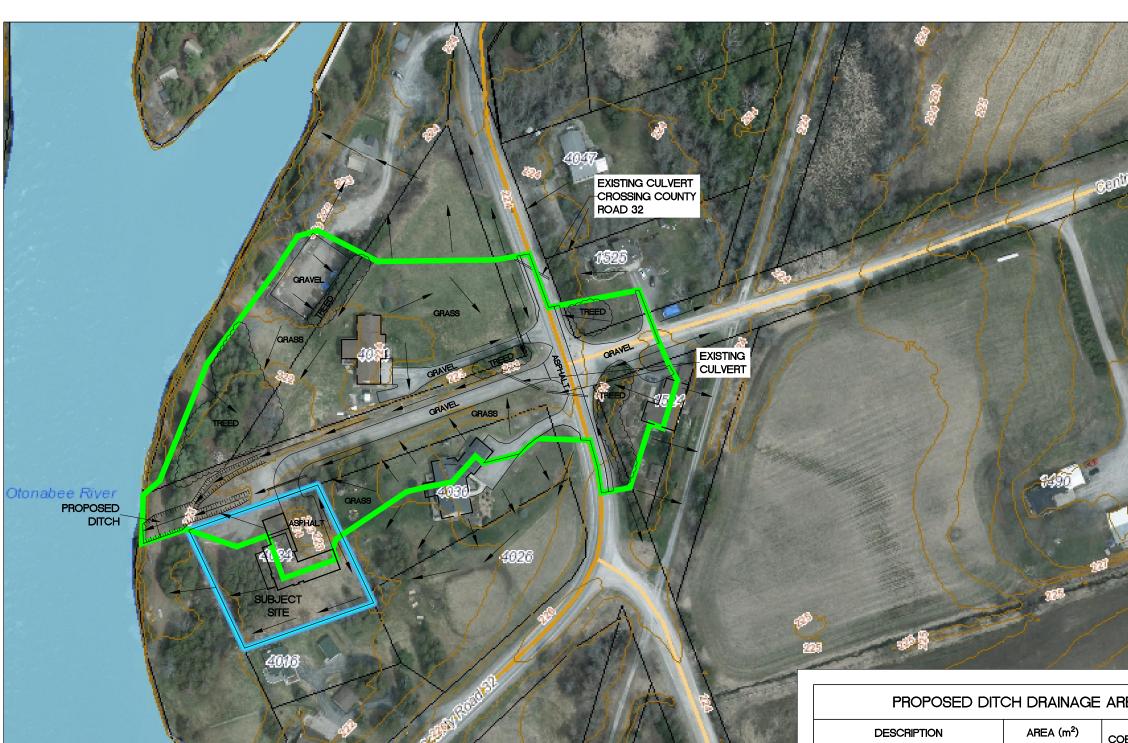
PATERSON & CARRINGTON

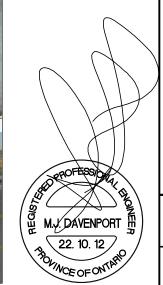
M.J. DAVENPORT

4034 CENTRE ROAD, BOLTONS CORNERS LOT 10 CONCESSION IX TOWNSHIP OF DOURO TOWNSHIP OF DOURO DUMMER COUNTY OF PETERBOROUGH

DRAINAGE AREA PLAN

DESIGNED BY: M. J. DAVENPORT	1: 1500	
DRAWN BY: J. CLARK		
OCTOBER, 2022	5883-03	
PROJECT NO.: 21-D-5883		





PROPOSED DITCH DRAINAGE AREA SUMMARY

DESCRIPTION	AREA (m²)	RUNOFF COEFFICIENT 'C'	COVERAGE (%)
IMPERVIOUS AREA	2,104.00	0.90	12.53
GRASS LANDSCAPED AREA	9,937.00	0.25	59.17
GRAVEL AREA	2,285.00	0.80	13.61
TREED AREA	2,467.00	0.25	14.69
TOTAL	16,793.00	0.41	100.00

